# EVALUATION OF THE EFFECTS OF SURFACE ROUGHNESS ON DISCHARGE COEFFICIENTS OF THE BROAD CRESTED WEIR

 $\mathbf{B}\mathbf{y}$ 

#### AHMED KHALID SALEH ATIA



Thesis Submitted in Partial Fulfillment as the Requirement for the Master of Science in Water Resources by Mix Mode in the Faculty of Engineering and Technology Infrastructure

**IUKL** 

2017

Abstract of thesis presented to the senate of Infrastructure University Kuala Lumpier in partial fulfillment of the requirement for the degree of Master of Science in Water Resources

EVALUATION OF THE EFFECTS OF SURFACE ROUGHNESS ON DISCHARGE
COEFFICIENTS OF THE BROAD CRESTED WEIR

# AHMED KHALID SALEH ATIA May2017

Chair: Assoc. Prof.Dr. Manal Mohsen Abood

Faculty: Faculty of Engineering and Technology Infrastructure IUKL.

Broad-crested weirs are one of the most widely used hydraulic structures in irrigation and drainage systems in natural streams where it is necessary to measure a wide range of discharges. The use of laboratory materials in hydraulic engineering gives a more accurate estimation of the discharge coefficients for broad-crested weirs with a rough crest surface.

For this research, a rectangular broad crested weir was investigated and studied through three models of length value and three roughness sizes ;5, 10 and 20 cm each with two heights 15cm and 25 cm, within different lengths 30, 60 and 90 cm each. The aim of this study was to determine the effects of the surface roughness sizes and its various heights on the discharge coefficients of the broad crested weir.

The results obtained from each situation show that increase in size of surface roughness lead to decrease of discharge value. Also increase of length and height of weir lead to decrease of discharge value due to the head loss caused by an increased surface resistance force.

There is positive relationship between H/P and discharge coefficients while the relationship between weir height and discharge coefficient is a strong negative relationship, while for the length and size of surface roughness, the relationship is

weak in range of thickness of  $0.0169 \le H/W \le 0.1004$  and in the large range of weir height (extreme 0.0407 < H/P < 0.401) while the range of H/Ks was  $0.0005 \le H/Ks \le 0.0601$ . As for the range of H/L, it was 0.011 < H/L < 0.2008.

#### **ACKNOWLEDGEMENT**

The first and foremost to be praised, for my success in completing this dissertation and to whom I am always indebted to and reliant upon, is ALLAH, who has best owed upon me power and the much needed inspiration and knowledge.

This research had been an uneasy journey in the wilderness of Scientific Knowledge. My research study would have not been completed without the many explorations, assessments, professional advices and encouragement from my supervisor. In addition to that, I am also sincerely grateful for the enthusiasm, support and prayers from my immediate family and friends; who was my pillar of strength, without whom it would have been impossible to accomplish this degree.

I would also wish to record my warmest appreciation to my supervisor who was a constant source of inspiration and strength; Associate Professor Dr. Manal Mohsen Abood for her continuous assessment, encouragement, and guidance.

Many thanks also go to the IUKL Centre for Postgraduate Studies and its academicians, including its remarkable secretary Miss Rosinah. More thanks to the librarians and to all other efficient service departments, who assisted me accordingly during my research.

Finally, love to my beloved father, mother, brother and sisters.

#### **APPROVAL**

This thesis was submitted to Senate of Infrastructure University Kuala Lumpier (IUKL) and has been accepted as partial fulfilment of the requirement for the degree of Master of Science in Water Resources. The members of the thesis Examination Committee were as follows:

## Dr .A.B.M. AMRUL KAISH , PHD

Position

: Lecturer

Faculty

: Faculty of Engineering and Technology Infrastructure

Infrastructure University Kuala Lumpier (IUKL)

(Chairman)

#### NOR AZIDAWATI BINTI HARON

Position

: Lecturer

Faculty

: Faculty of Engineering and Technology Infrastructure

Infrastructure University Kuala Lumpier (IUKL)

(Internal Examiner)

#### **AZLINDA BINTI SAADON**

Position

: Lecturer

Faculty

: Faculty of Engineering and Technology Infrastructure

Infrastructure University Kuala Lumpier (IUKL)

(Internal Examiner)

-

Assoc. Prof. Dr Manal Mohsen Abood

Director

Centre for Postgraduale Studies
Infrastructure University Kuala Lumpur (IUKL)

Assoc. Prof. Dr Manal Mohsen Abood

Director

Center for Postgraduate Studies

Infrastructure University Kuala Lumpier (IUKL)

Date 3115/17

#### **DECLARATION**

I declare that the thesis is my original work except for quotations and citations which have been duly acknowledged. I also declare that it has not been previously, and is not concurrently, submitted for any other degree at Infrastructure University Kuala Lumpur or at any other institution.

Signature

Name

: Ahmed Khalid Saleh Atia : 30-5-2017

Date

TAB	LE OF	CONTENTS	Page
ABS	TRACT		ii
ACK	KNOWL	EDGEMENT	iv
APP	ROVAL		$\mathbb{V}$
	CLARAT		vi
		CONTENTS	vii
	Γ OF TA		ix
	r of fi		X::
LIS	I OF AL	BBREVIATIONS	xii
CIL	APTER		
1		RODUCTION	1
	1.1	General	1
	1.2	Types of Weirs	2
	1.3	Statement of Problem	3
	1.4	Aim of Study	3
	1.5	Objectives of the Study	4
	1.6 1.7	Scope of the Study Significance of the Study	4
2	LITE	CRATURE REVIEW	6
	2.1	Introduction	6
	2.2	Effects of Surface Roughness on Discharge Coefficients	11
	2.3	Effect of Stream Flow Rate on Discharge Coefficients	13
	2.4	Broad Crested Weir Hydraulics	15
	2.5 2.6	Discharge Coefficients Weir Roughness Expressed by the Friction Coefficient	17 18
3	MET	THODOLOGY	29
	3.1	Introduction	29
	3.2	Materials and Equipment	29
		3.2.1 Flume- Open Channel	29
		3.2.2 Rectangular Broad-crested Weir	30
		3.2.3 Flow Measurement Device (Flow Converter 713)	30
	3.3	3.2.4 Gravel Particles with Various Surface Roughness's Experimental Setup	31 32
	3.4	Experimental Setup  Experimental Procedure	33
	3.5	Theoretical Approach	35
	3.6	Static Program Data Analysis	36
	3.7	Chart Flow of Work	37
	3.8	Gantt Chart of Research	38

3.8 Gantt Chart of Research

4	RESU	LTS, ANA	ALYSIS AND DISCUSSION	39
	4.1 4.2 4.3 4.4 4.5 4.6 4.7 4.8 4.9 4.10	Variation Effect of Effect of Effect of Effect of Formulat Simulation	of the Discharge with Total Head (Height 15 cm) of the Discharge with Total Head (Height 25 cm) Weir Length and Variation of Cd (Height 15 Cm) Weir Length and Variation of Cd (Height 25 Cm) Weir Heights and Variation of Cd (H/P) Values Weir Surface Roughness and Variation of Cd (H/Ks) ion of Discharge Coefficient Models on and Prediction Comparative son of Cd versus H/L, H/P and H/Ks Comparison of Cd versus H/L with Muralidhar	39 39 42 45 47 50 51 53 56 58
		4.10.1	Comparison of Cd versus H/P with Rehbock	60
		4.10.2	Comparison of Cd versus H/Ks with Jalil Equation	61
		4.10.5	Comparison of Ca volues 11/10 with sain Equation	01
5	CON	CLUSION		62
	5.1	Conclus	ion	62
	5.2	Recomm	nendations	64
	REF	ERENCES	S	65
			*	

### LIST OF TABLES

Table 2.1:	Broad-crested Weir Limits	16
Table 2.2	Cross section distance from weir crest (cm)	24
Table 3.1:	Flow Converter 713 Specifications	31
Table 3.2:	Details of the Broad Crested Weir Models Experimental Program	34
Table 4.1:	Different Size of Ks with Variation of the Discharge at Height 15cm	40
Table 4.2:	Different Size of Ks with Discharge Variation at Height 25 cm	43
Table 4.3:	Different Value of the Cd and H/L for Different Ks at Height 15cm	45
Table 4.4:	Different Values of the Cd and $H/L$ for Different Ks at Height 25cm	48
Table 4.5:	Descriptive Analysis of the Calculated Coefficient of Discharge	53
Table 4.6:	The Correlation between the Dependent Variable Cd	54
Table 4.7:	The Regression Models Analysis	55
Table 4.8:	Model Summary	56
Table 4.9:	Model Summary Coefficients	56
Table 4.10:	Discharge Coefficient versus H/L, H/P and H/Ks Values.	58

# LIST OF FIGURES

Figure 2.1:	Scheme of Weir, Flow and Notation of Variables	9
Figure 2.2:	Broad Crested Weir Model	12
Figure 2.3:	Variation of the Discharge with Total Head for Different Weir Length	12
Figure 2.4:	Variation of the Discharge with Total Head for Different Roughness	13
Figure 2.5:	Variation of the discharge coefficient and H/L for different Ks	13
Figure 2.6:	Schematic Diagram of Cramp Weir Model	14
Figure 2.7:	Conditions at a Broad-Crested Weir	16
Figure 2.8:	Head-Discharge Relations of the Weir for Individual Variants	21
Figure 2.9:	Approximation of Cd(h) Related to the Grain Size ks = d50	21
Figure 2.10:	Plan View of Experimental Setup (Unit: m)	23
Figure 2.11:	Stage-Discharge Relation on Weir for Different Roughness Sizes	24
Figure 2.12:	Discharge Coefficient Against Relative Roughness D50/Hw	25
Figure 2.13:	Experimental Setup Used for Conducting the Experiment	25
Figure 2.14:	Flow Pattern above a Broad-Crested Weir	27
Figure 2.15:	Cd versus H1/L for Varying P and b and Constant z and (L)	27
Figure 3.1:	Flume- Open Channel Water	29
Figure 3.2:	Plastic (PVC) Rectangular Crest Weirs	30
Figure 3.3:	Flow Converter 713	30
Figure 3.4:	Gravel Particles	32
Figure 3.5:	Shaker with Shaker Multi Sieves	32
Figure 3.6:	Profile of a Broad-Crested Rectangular Weir	35
Figure 3.7:	Research Flowchart	37
Figure 3.8:	Gantt-Chart of Research	38
Figure 4.1:	Ks with Variation of the Discharge at Length 30 and Height 15 cm	41
Figure 4.2:	Ks with Variation of the Discharge at Length 60 and Height 15 cm	42
Figure 4.3:	Ks with Variation of the Discharge at Length 90 and Height 15 cm	42
Figure 4.4:	Ks with Variation of the Discharge at Length 30 Cm & Height 25 Cm	44
Figure 4.5:	Ks with Variation of the Discharge at Length 60 Cm & Height 25 Cm	44
Figure 4.6:	Ks with Variation of the Discharge at Length 90 Cm & Height 25 Cm	44
Figure 4.7:	Variation of the Cd and H/L for Ks at Length 30 Cm & Height 15 Cm	46
Figure 4.8:	Variation of the Cd and H/L for Ks at Length 60 Cm & Height 15 Cm	46
Figure 4.9:	Variation of the Cd and H/L for Ks at Length 90 Cm & Height 15 Cm	47
Figure 4.10:	Variation of the Cd and H/L for Ks at Length 30 Cm & Height 25 Cm	49
Figure 4.11:	Variation of the Cd and H/L for Ks at Length 60 Cm & Height 25 Cm	49
Figure 4.12:	Variation of the Cd and H/L for Ks at Length 90 Cm & Height 25 Cm	49

Figure 4.13:	Variation of the Cdand H/P for $Ks = 0$ at Height 15 and 25 Cm	50
Figure 4.14:	Variation of the Cd and H/P for $Ks = 10$ at Height 15 And 25 Cm	50
Figure 4.15:	Variation of the Cd and H/H for $Ks = 20$ at Height 15 and 25 Cm	51
Figure 4.16:	Variation of the Cd and H/Ks for Length 30 cm at Height 15 cm	52
Figure 4.17:	Variation of the Cd and H/Ks for Length 60 cm at Height 15 cm	52
Figure 4.18:	Variation of the Cd and H/Ks for Length 90 cm at Height 15 cm	52
Figure 4.19:	Discharge Coefficient Prediction and Simulation of H/P	57
Figure 4.20:	Discharge Coefficient Prediction and Simulation of H/L	57
Figure 4.21:	Discharge Coefficient Prediction and Simulation of H/Ks	57
Figure 4.22:	Cd Comparison versus H/L with (Rao & Muralidhar, 1963)	59
Figure 4.23:	Cd Comparison versus H/P with (Rehbock, 1929)	60
Figure 4.24:	Experimental Cd Comparison versus Prediction Cd (Jalil et al., 2014)	61

## LIST OF ABBREVIATIONS

Abbreviation	Description
A	Flow area in section of weir
В	Width of the crest of the dam or weir
cf	Friction coefficient
C	Chézy coefficient
C1	Numerical coefficient of broad-crested weir flow
Cd	Discharge coefficient
C dc	Discharge coefficient over the crest
d	Water depth
D	Hydraulic diameter
f	friction factor
Fr	Froude number
g	Gravitational acceleration
h	Upstream water level above weir crest
L	Water level over weir
L	Water level over weir
H	Total energy head
Ks	Roughness size of crest
M	Weir slope
n	Manning coefficient
p	Pressure
P	Weir height
q	Discharge per unit width
Q	Total discharge
$Q_{th}$	Theoretical discharge over the weir
Qact	Actual discharge over the weir
V	flow velocity
W	Width of flume

#### **CHAPTER 1**

#### INTRODUCTION

#### 1.1 General

In order to measure the level of water in an upstream, hydraulic structures are usually installed in the rivers and open channels. The level of water in the upstream will then be used in estimating the discharge (Boiten, 1993). Weirs, for example are types of hydraulic structures that act as obstructions in rivers and open channels, and are used to measure the amount of discharge. Two main methods are often used to measure the flow in these hydraulic structures. The first method is known as the velocity areas method and the second one is the hydraulic head method. In the first method, the flow discharge is usually measured by finding the local velocity of the areas that have been affected, while the second method is used to determine the flow discharge by measuring the available difference in the hydraulic head through the use of the flow reach. In order to conclude these methods, empirical equations are usually used between the flow head and discharge. At times, these measurements are dissimilar in value from those obtained in the field.

Weirs are simple devices used to measure discharge and control the flow in open channels, like canals and rivers. They are used as a form of a measurement tool for discharges during the task of monitoring of floods at respective stations (in the rivers and canals). Weirs are small overflow-type dams, commonly, used to raise the water levels of a river or a stream, thus, causing a large change of water levels behind them. Many researchers have studied the head discharge relations for flows over the sharp-crested and broad-crested weirs with a simple cross sectional shape, like a rectangular, triangular, trapezoidal, truncated triangular, and others (Seyed Hooman Hoseini, Shayegan, & Afshar, 2013).

Broad crested weirs are some of the widely used hydraulic structures that individuals use in measuring the rate of flow in irrigations, rivers, open channels and drainage systems.

A broad-crested weir is a structure created with a flat crest in such a way that the length of the crest is larger when compared to the thickness of the flow (Gonzalez &

#### REFERENCES

- Ali, S., & Uijttewaal, W. (2009). The form drag due to vegetated weir-like obstacles interpreted as expansion losses. Paper presented at the Water engineering for sustainable environment: 33rd IAHR congress; 9-14 August 2009, Vancouver, British Columbia, Canada.
- Ali, S., & Uijttewaal, W. (2013). Flow resistance of vegetated oblique weir-like obstacles during high water stages. *Hydrology and Earth System Sciences Discussions*, 10 (4), 2013.
- Azimi, A. H., & Rajaratnam, N. (2009). Discharge characteristics of weirs of finite crest length. *Journal of hydraulic engineering*, 135(12), 1081-1085.
- Badr, K., & Mowla, D. (2015). Development of rectangular broad-crested weirs for flow characteristics and discharge measurement. *KSCE Journal of Civil Engineering*, 19(1), 136-141.
- Boiten, W. (1993). Flow-measuring structures. Flow Measurement and Instrumentation, 4(1), 17-24.
- Boiten, W., & Pitlo, R. H. (1982). The V-shaped broad-crested weir. *Journal of the Irrigation and Drainage Division*, 108(2), 142-160.
- Bos, M. G. (1989). Discharge measurement structures. *Discharge measurement structures*.
- Castro-Orgaz, O. (2010). Steady open channel flows with curved streamlines: the Fawer approach revised. *Environmental fluid mechanics*, 10(3), 297-310.
- Coleman, N. M., Kaktins, U., & Wojno, S. (2016). Dam-Breach hydrology of the Johnstown flood of 1889–challenging the findings of the 1891 investigation report. *Heliyon*, 2(6), e00120.

-LIBRARY INFRASTRUCTURE UNIVERSITY KUALA LUMPUR - (STAT)

- Emiroglu, M. E., & Kaya, N. (2011). Discharge coefficient for trapezoidal labyrinth side weir in subcritical flow. *Water resources management*, 25(3), 1037-1058.
- Ferrari, A. (2010). SPH simulation of free surface flow over a sharp-crested weir. Advances in Water Resources, 33(3), 270-276.
- Fritz, H. M., & Hager, W. H. (1998). Hydraulics of embankment weirs. *Journal of Hydraulic Engineering*, 124(9), 963-971.
- Garcia, M. H. (2008). Sediment transport and morphodynamics. *Sedimentation* engineering: *Processes, measurements, modeling, and practice*(110).
- Ghobadian, R., Fattahi, A., Farmanifard, M., & Ahmadi, A. (2013). Effect of crest roughness on flow characteristics over circular weirs. *Civil Engineering Infrastructures Journal*, 46(2), 199-207.
- Göğüş, M., Defne, Z., & Özkandemir, V. (2006). Broad-crested weirs with rectangular compound cross sections. *Journal of irrigation and drainage engineering*, 132(3), 272-280.
- Gonzalez, C. A., & Chanson, H. (2007). Experimental measurements of velocity and pressure distributions on a large broad-crested weir. *Flow Measurement and Instrumentation*, 18(3), 107-113.
- GOodarzi, E., Farhoudi, J., & Shokri, N. (2012). Flow characteristics of rectangular broad-crested weirs with sloped upstream face. *Journal of Hydrology and Hydromechanics*, 60(2), 87-100.
- Grupp, I. L., French, J. F., & Matlib, M. A. (1987). Benzodiazepine Ro 5-4864 increases coronary flow. *European journal of pharmacology*, 143(1), 143-147.
- Hager, W. H., & Schwalt, M. (1994). Broad-crested weir. *Journal of irrigation and drainage engineering*, 120(1), 13-26.

-LIBRARY MITASTRUCTURE DRIVERSITY KUALA LUMPUR (S.P.O.)

- Harrison, A., Montes, S., Manning, A., Halliwell, A., Williams, J., Markland, E., . . . Abbott, M. (1969). The streamlined broad-crested weir. *Proceedings of the Institution of Civil Engineers*, 42(4), 575-599.
- Hoseini, S. H., Jahromi, S. H. M., & Vahid, M. S. R. (2013). Determination of discharge coefficient of rectangular broad-crested side weir in trapezoidal channel by CFD. *International Journal of Hydraulic Engineering*, 2(4), 64-70.
- HOSEINI, S. H., Shayegan, A., & Afshar, H. (2013). Study of Flow over a Triangular Broad-Crested Weir. In T. Islamic Azad University of Central Tehran Branch, Iran. (Ed.).
- Hussein, H. H., Juma, I. A., & Shareef, S. J. Improving the Hydraulic Performance of Single Step Broad-Crested Weirs.
- Hussien, N. J. (2014). Experimental Study of Height and Surface Roughness Effects of Crump Weirs on Over Flow Characteristics. *Journal of Babylon University/Engineering Sciences*, 22.
- Jalil, S. A., Ibrahim, S. S., & Jafer, R. A. (2014). Surface Roughness Effects on Discharge Coefficient of Broad Crested Weir. Research Journal of Applied Sciences, Engineering and Technology, 7(24), 5227-5233.
- Kiselev, P. (1972). Handbook of Hydraulic Calculations [in Russian], 1~ nergiya.

  Moscow (1972).
- Kothyari, U. C., Garde, R. C. J., & Ranga Raju, K. G. (1992). Temporal variation of scour around circular bridge piers. *Journal of Hydraulic Engineering*, 118(8), 1091-1106.
- Maghrebi, M., Alizadeh, S., & Lotfi, R. (2012). Numerical Simulation of Flow Over Rectangular Broad Crested Weir (Real case study). Paper presented at the The First International Conference on Dams and Hydropower.

- Mahmodinia, S., Javan, M., & Eghbalzadeh, A. (2014). The effects of side-weir height on the free surface turbulent flow. KSCE Journal of Civil Engineering, 18(7), 2244-2251.
- Munson, B. R., Young, D. F., & Okiishi, T. H. (1994). Fundamentals of fluid dynamics.
- Nguyen, B. (2006). Flow over oblique weirs. TU Delft, Delft University of Technology.
- Othman, K. I., Chilmeran, T. A., & Al-Hafith, I. A. (2011). Effect of Size and Surface Roughness of Cylindrical Weirs on Over Flow Characteristics. *Al-Rafadain Engineering Journal*, 19(2).
- Pařílková, J., Říha, J., & Zachoval, Z. (2012). The influence of roughness on the discharge coefficient of a broad-crested weir. *Journal of Hydrology and Hydromechanics*, 60(2), 101-114.
- Ramamurthy, A. S., Tim, U. S., & Rao, M. (1988). Characteristics of square-edged and round-nosed broad-crested weirs. *Journal of Irrigation and Drainage Engineering*, 114(1), 61-73.
- Rao, N. G., & Muralidhar, D. (1963). Discharge characteristics of weirs of finite-crest width. *La Houille Blanche*(5), 537-545.
- Rehbock, T. (1929). Discussion of precise weir measurements by EW Schoder and KB Turner. *Trans ASCE*, *93*, 1143-1162.
- Salmasi, F., Poorescandar, S., Dalir, A. H., & Zadeh, D. F. (2011). Discharge relations for rectangular broad-crested weirs. *Journal of Agricultural Sciences*, 17, 324-336.
- Salmasi, F., Pooreskandar, S., HosseinzadeDalir, A., & Farsadizade, D. (2011).

  Discharge relations for rectangular broad-crested weirs. *Journal of Agricultural Sciences*, 17, 324-336.

- Salmasi, F., Yıldırım, G., Masoodi, A., & Parsamehr, P. (2013). Predicting discharge coefficient of compound broad-crested weir by using genetic programming (GP) and artificial neural network (ANN) techniques. *Arabian Journal of Geosciences*, 6(7), 2709-2717.
- Sargison, J. E., & Percy, A. (2009). Hydraulics of broad-crested weirs with varying side slopes. *Journal of Irrigation and Drainage Engineering*, 135(1), 115-118.
- Schlichting, H. T., & Truckenbrodt, E. A. (1979). *Aerodynamics of the Airplane*: McGraw-Hill Companies.
- Van Rijn, L. C. (1990). Principles of fluid flow and surface waves in rivers, estuaries, seas and oceans (Vol. 11): Aqua Publications Amsterdam, The Netherlands.
- Zachoval, Z., Knéblová, M., Roušar, L., Rumann, J., & Šulc, J. (2014). Discharge coefficient of a rectangular sharp-edged broad-crested weir. *Journal of Hydrology and Hydromechanics*, 62(2), 145-149.
- Zachoval, Z., & Roušar, L. (2015). Flow structure in front of the broad-crested weir.

  Paper presented at the EPJ Web of Conferences.

